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Table 1 Percentage Increase in Net Bearing Pressure and Skin Friction of Soils
(*Clause* 6.3.5.2)

Sl No. (1)	Soil Type (2)	Percentage Increase Allowable (3)
i)	*Type A:* Rock or hard soils	50
ii)	*Type B:* Medium or stiff soils	25
iii)	*Type C:* Soft soils	0

NOTES

1 The net bearing pressure shall be determined in accordance with IS 6403 or IS 1888.

2 Only corrected values of N shall be used.

3 If any increase in net bearing pressure has already been permitted for forces other than seismic forces, the increase in allowable bearing pressure, when seismic force is also included, shall not exceed the limits specified above.

4 The desirable minimum corrected field values of N shall be as specified below:

Seismic Zone	*Depth (m) below Ground Level*	*N Values*	*Remarks*
III, IV and V	≤ 5 ≥10	15 25	For values of depths between 5 m and 10 m, linear interpolation is recommended
II	≤ 5 ≥10	10 20	

If soils of lower N values are encountered than those specified in the table above, then suitable ground improvement techniques shall be adopted to achieve these values. Alternately, deep pile foundations should be used, which are anchored in stronger strata, underlying the soil layers that do not meet the requirement.

5 Piles should be designed for lateral loads neglecting lateral resistance of those soil layers (if any), which are liable to liquefy.

6 Indian Standards IS 1498 and IS 2131 may be referred for soil notation, and corrected N values shall be determined by applying correction factor C_N for effective overburden pressure σ'_{vo} using relation $N = C_N N_1$, where $C_N = \sqrt{P_a/\sigma'_{vo}} \leq 1.7$, P_a is the atmospheric pressure and N1 is the uncorrected SPT value for soil.

7 While using this table, the value of N to be considered shall be determined as below:

a) Isolated footings — Weighted average of N of soil layers from depth of founding, to depth of founding plus twice the breadth of footing;

b) Raft foundations — Weighted average of N of soil layers from depth of founding, to depth of founding plus twice the breadth of raft;

c) Pile foundation — Weighted average of N of soil layers from depth of bottom tip of pile, to depth of bottom tip of pile plus twice the diameter of pile;

d) Group pile foundation — Weighted average of N of soil layers from depth of bottom tip of pile group, to depth of bottom tip of pile group plus twice the width of pile group; and

e) Well foundation — Weighted average of N of soil layers from depth of bottom tip of well, to depth of bottom tip of well plus twice the width of well.

Table 2 Classification of Types of Soils for Determining Percentage Increase in Net Bearing Pressure and Skin Friction
(*Clause* 6.3.5.2)

Sl No. (1)	Soil Type (2)	Remarks (3)
i)	**Type A** Rock or hard soils	Well graded gravel (GW) or well graded sand (SW) both with less than 5 percent passing 75 mm sieve (Fines) Well graded gravel — sand mixtures with or without fines (GW-SW) Poorly-graded sand (SP) or Clayey sand (SC), all having N above 30 Stiff to hard clays having N above 30, where N is corrected standard penetration test value
ii)	**Type B** Medium or stiff soils	Poorly graded sands or poorly graded sands with gravel (SP) with little or no fines having N between 10 and 30 Stiff to medium stiff fine-grained soils, like silts of low compressibility (ML) or clays of low compressibility (CL) having N between 10 and 30
iii)	**Type C** Soft soils	All soft soils other than SP with N<10. The various possible soils are: Silts of intermediate compressibility (MI); Silts of high compressibility (MH); Clays of intermediate compressibility (CI); Clays of high compressibility (CH); Silts and clays of intermediate to high com-pressibility (MI-MH or CI-CH); Silt with clay of intermediate compressibility (MI-CI); and Silt with clay of high compressibility (MH-CH).
iv)	**Type D** Unstable, collapsible, liquefiable soils	Requires site-specific study and special treatment according to site condition (*see* **6.3.5.3**)

Table 3 Seismic Zone Factor *Z*
(*Clause* 6.4.2)

Seismic Zone (1)	II (2)	III (3)	IV (4)	V (5)
Factor Z	0.10	0.16	0.24	0.36

6.4.3 Effects of design earthquake loads applied on structures can be considered in two ways, namely:

a) Equivalent static method, and

b) Dynamic analysis method.

In turn, dynamic analysis can be performed in three ways, namely:

1) Response spectrum method,

2) Modal time history method, and

3) Time history method.

In this standard, Equivalent Static Method, Response Spectrum Method and Time History Method are

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